

# Memorandum

**Date:** August 30, 2022

**To:** Mr. Barti Oumarou, Basin Engineer, Wisconsin Department of Natural Resources

**From:** Mike Gerbitz, Ryan Holzem, Christine Wood - Donohue

**Re: Capacity Re-Rate Request**  
Heart of the Valley Metropolitan Sewerage District, Kaukauna, WI  
Donohue Project Number 14649

The Heart of the Valley Metropolitan Sewerage District (HOVMSD) owns and operates an advanced water reclamation (WRRF or Facility). This Memorandum asks the Department to increase the Facility’s 5-day biochemical oxygen demand (BOD) and ammonia (NH<sub>3</sub>) capacities. Although the WRRF possesses more total suspended solids (TSS) and total phosphorus (TP) capacity, this submittal does not request more capacity for these parameters.

### Current Rated Capacities

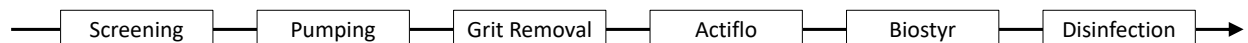
**Table 1** shows the original maximum month WDNR-approved flow and loading capacities for the WRRF. The design calculations that arrived at these values are provided in **Appendix A**.

**Table 1:** Current Flow and Loading Capacities

Parameter	Maximum Month
Flow (mgd)	11.9
BOD (ppd)	14,651
TSS (ppd)	16,278
Ammonia (ppd)	1,920
TP (ppd)	407

### Liquid Train Configuration

The forward flow consists of screening, grit removal, sand-ballasted primary settling (Actiflo), fixed-film biological treatment, and chlorine disinfection. The biological treatment process is a biological aerated filter (BAF or, its specific trade name, Biostyr). **Figure 1** shows a simple liquid train flow schematic.



*Figure 1.* Liquid Treatment Train Schematic.

The most recent WPDES Permit regulating the Facility includes total maximum daily load (TMDL) limits for TSS and TP. An active construction project is adding tertiary effluent filtration. The filtration system

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will enhance TSS and TP removal and enhance the Facility’s ability to comply with these TMDL limits. **Figure 2** shows the liquid train with tertiary filtration.

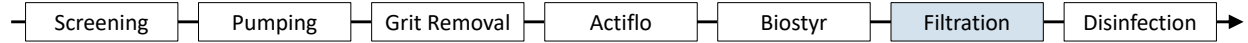


Figure 2. Liquid Treatment Train Schematic with Effluent Filtration.

Solids Train Configuration

Solids stabilization consists of gravity thickening (GT), dissolved air flotation thickening (DAFT), autothermal aerobic digestion (ATAD) with post-ATAD nitrification, and liquid solids storage. **Figure 3** shows a simple solids train schematic. Overflow and underflow sidestreams from the thickening processes (GT and DAFT) return to the liquid treatment train upstream of primary settling (Actiflo).

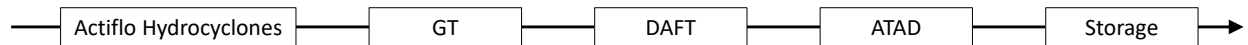


Figure 3. Solids Stabilization Schematic.

BOD and NH3 Removal Processes

Two existing unit processes remove BOD and NH3 from the forward flow: Actiflo and Biostyr. In the future, tertiary filtration will remove secondary effluent TSS and the BOD associated with those solids. **Figure 4** highlights the processes that remove BOD and NH3.

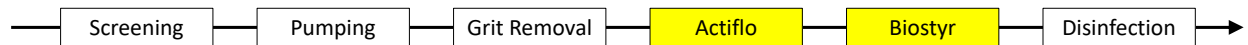


Figure 4. BOD- and NH3-Removing Processes

Original BOD and NH3 Design Assumptions

The Biostry process includes eight separate treatment cells. The media in these eight cells occupies 86,480 cubic feet (Figure 5).

**Biostyr Volume**

Parameter	Units	Value
Area per Cell	sf	940
Cell Height	ft	11.5
Number of Cells	#	8
Total Volume	cf	86,480
	kcf	86.48

Figure 5. Biostry Media Volume

The original design basis made several assumptions that defined the Facility’s BOD and NH3 treatment capacities: [1] Actiflo would remove 50% of the influent BOD, [2] Actiflo would remove 0% of the influent NH3, [3] Biostyr’s eight cells could treat 84.71 pounds per day (ppd) of BOD per 1000 cubic feet (kcf) of media volume , [4] Biostyr’s eight cells could treat 24.42 ppd-NH3/kcf, and [5] sidestream

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loadings would increase Biostyr loadings by 10%. The original design only considered sidestream loadings for ammonia. The calculations below show how these assumptions defined the present-day BOD and NH<sub>3</sub> loading capacities.

### BOD Removal

Equation 1 represents the total forward flow BOD treatment capacity for the WRRF.

$$BOD_{WRRF} = BOD_{Actiflo} + BOD_{Biostyr} \quad (1)$$

where

$BOD_{WRRF}$	BOD design capacity of WRRF
$BOD_{Actiflo}$	BOD removal from Actiflo at WRRF design capacity
$BOD_{Biostyr}$	BOD removal from Biostyr at WRRF design capacity

Equation 2 represents the total BOD capacity for Actiflo. Equation 3 represents the total BOD capacity for Biostyr based on unit volume loadings.

$$BOD_{Actiflo} = \%Removal \times BOD_{WRRF} \quad (2)$$

$$BOD_{Biostyr} = BOD_{kcf} \times V_{kcf} \quad (3)$$

where

$BOD_{kcf}$	Design BOD loading on a unit volume basis
$V_{kcf}$	Media volume

The above equations can be arranged to provide Equation 4.

$$BOD_{WRRF} = (BOD_{kcf} \times V_{kcf}) / (1 - \%Removal) \quad (4)$$

Assuming Actiflo will remove 50% of the influent BOD and the eight Biostyr cells will treat 84.71 ppd-BOD/kcf, Equation 4 yields a BOD capacity of 14,651 ppd-BOD.

$BOD_{WRRF} = (84.71 \text{ ppd-BOD/kcf} \times 86.58 \text{ kcf}) / (0.5) = 14,651 \text{ ppd-BOD} \quad (5)$
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### Ammonia Removal

Equation 6 represents the total forward flow NH<sub>3</sub> treatment capacity for the WRRF.

$$BOD_{WRRF} = NH3_{Actiflo} + NH3_{Biostyr} \quad (6)$$

where

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$NH3_{WRRF}$	NH3 design capacity of WRRF
$NH3_{Actiflo}$	NH3 removal from Actiflo at WRRF design capacity
$NH3_{Biostyr}$	NH3 removal from Biostyr at WRRF design capacity

Equation 7 represents the total NH3 capacity for Actiflo. Equation 8 represents the total NH3 capacity for Biostyr based on unit volume loadings.

$$NH3_{Actiflo} = \%Removal \times NH3_{WRRF} \quad (7)$$

$$NH3_{Biostyr} = NH3_{kcf} \times V_{kcf} \quad (8)$$

where

$NH3_{kcf}$	Design NH3 loading on a unit volume basis
$V_{kcf}$	Media volume

The above equations can be arranged to provide Equation 9.

$$NH3_{WRRF} = (NH3_{kcf} \times V_{kcf}) / (1 - \%Removal) \quad (9)$$

Assuming Actiflo will remove 0% of the influent NH3, the eight Biostyr cells will treat 24.42 ppd-NH3/kcf, and 10% of the ammonia loading to Biostyr is attributed to an internal sidestream, Equation 9 yields a NH3 capacity of 1,902 ppd-NH3.

$$NH3_{WRRF} = (24.42 \text{ ppd-BOD/kcf} \times 86.58 \text{ kcf}) \times 90\% = 1,902 \text{ ppd-NH3} \quad (5)$$

### Full-Scale Stress Test

A full-scale, cold-weather stress test was conducted between February 7 and May 16, 2022. The purpose of the stress test was to investigate the true BOD and NH3 treatment capacity of the liquid train when influent wastewater is cold and treatment kinetics are the slowest.

Figure 5 shows where samples were collected. Samples collected upstream and downstream of Actiflo (Locations 2 and 3) revealed Actiflo's effective BOD and NH3 removal performance. Volumetric BOD and NH3 loadings to Biostyr were artificially increased by reducing the number of Biostyr cells in service. Samples collected at Locations 1 and 4 show the treatment performance of the Actiflo-Biostyr system with Biostyr operating at artificially elevated loadings.

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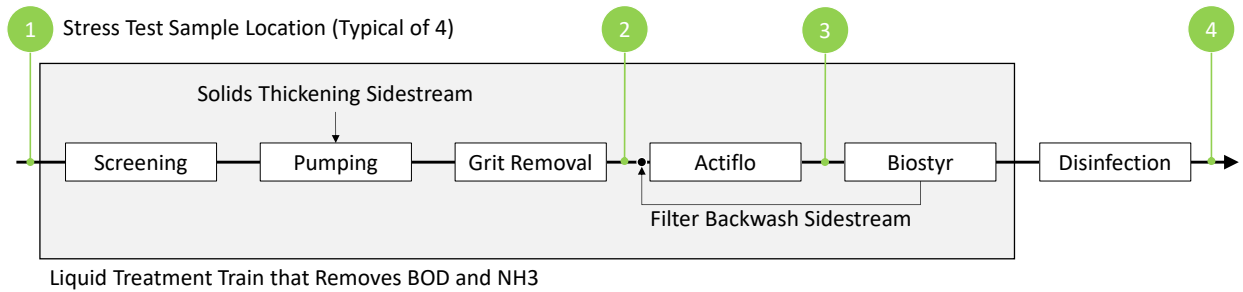


Figure 5. Stress Test Sample Locations

### Temperature

Influent wastewater temperatures were cold, ranging from 49 – 55°F. Elevated loadings occurred during some of the coldest temperatures.

### Actiflo

Figure 6 shows the measured BOD removal efficiencies. BOD removal exceeded that assumed in the original design, averaging 63%. The original design assumed 50%. All but one measurement exceeded the assumed design value.

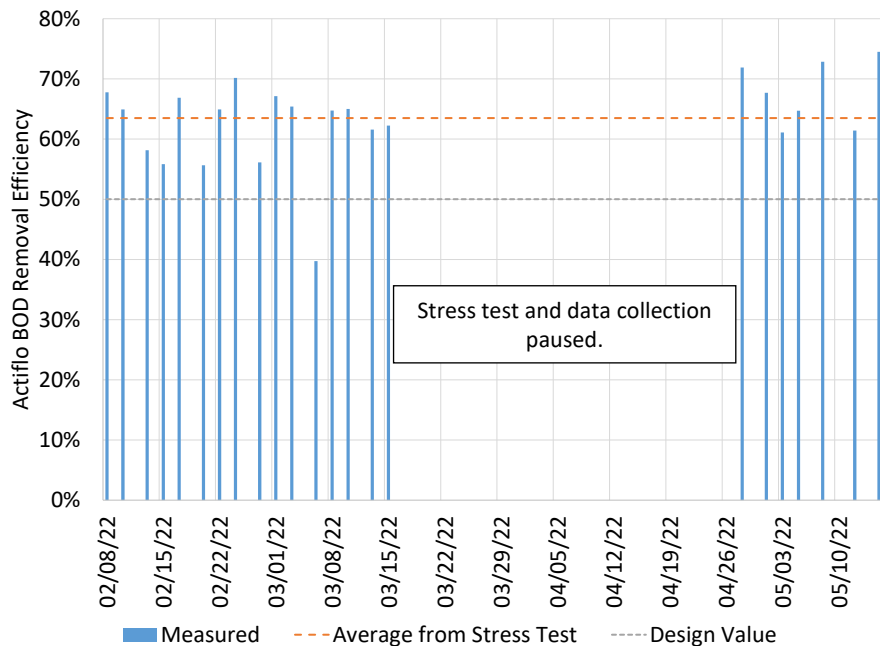


Figure 6. Measured Actiflo BOD Removal Efficiencies

Figure 7 shows the measured total Kjeldahl nitrogen (TKN) removal efficiencies. TKN is comprised of ammonia and organic nitrogen, representing the total nitrogen load that must be nitrified in the Biostyr process. Actiflow removal efficiencies were minimal and highly variable. Some of the values were less than

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zero, likely revealing effects of internal sidestreams. The original design assumed 0%. The average was 10%. Despite some good removal performance, the stress test confirmed that Actiflo is not a reliable TKN (ammonia) process.

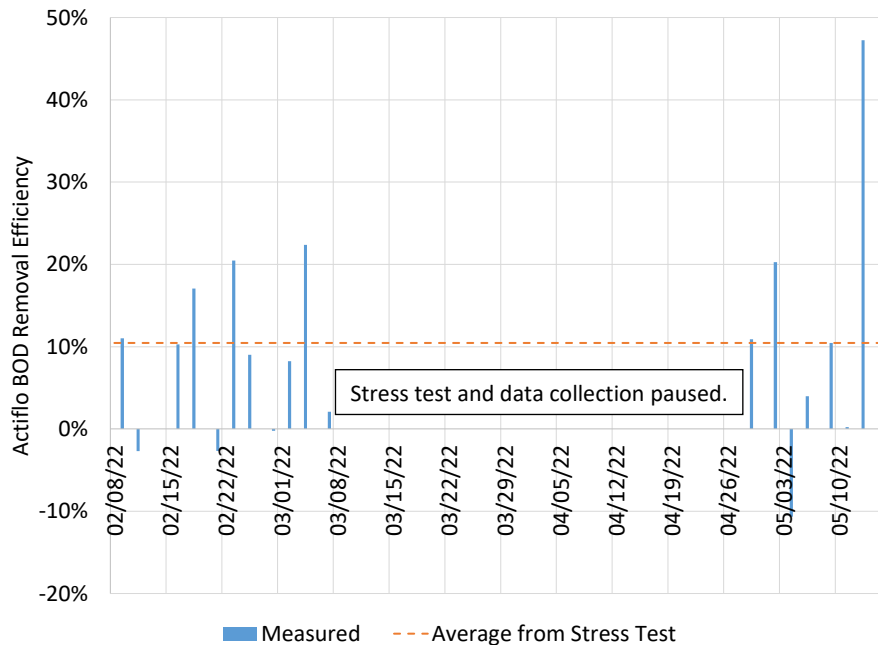


Figure 7. Measured Actiflo TKN (NH<sub>3</sub>) Removal Efficiencies

### Actiflo-Biostyr System

The original design assumed Actiflo would remove 50% of influent BOD and Biostyr would treat 84.71 ppd-BOD/kcf. In other words, the original design assumed the Actiflo-Biostyr process would treat a raw wastewater BOD loading of 169.42 ppd-BOD/kcf. Figure 8 shows raw wastewater BOD loadings per unit volume of Biostyr media in service versus final effluent BOD concentrations. Unit volume loadings far exceeded the design value (169.42 ppd-BOD/kcf) and, with only one exception, effluent BOD concentrations were well below the monthly average effluent limit.

The original design assumed Actiflo would remove 0% of influent NH<sub>3</sub>, internal sidestreams would contribute 10% of the Biostyr ammonia loading, and Biostyr would treat 24.42 ppd-NH<sub>3</sub>/kcf. In other words, the original design assumed the Actiflo-Biostyr process would treat a raw wastewater NH<sub>3</sub> loading of 21.98 ppd-NH<sub>3</sub>/kcf. Figure 9 shows raw wastewater NH<sub>3</sub> loadings per unit volume of Biostyr media in service versus final effluent NH<sub>3</sub> concentrations. Unit volume loadings far exceeded the design value (21.92 ppd-NH<sub>3</sub>/kcf) and effluent NH<sub>3</sub> was well below the most stringent ammonia effluent limit that occurs during the warmest months: June - September.

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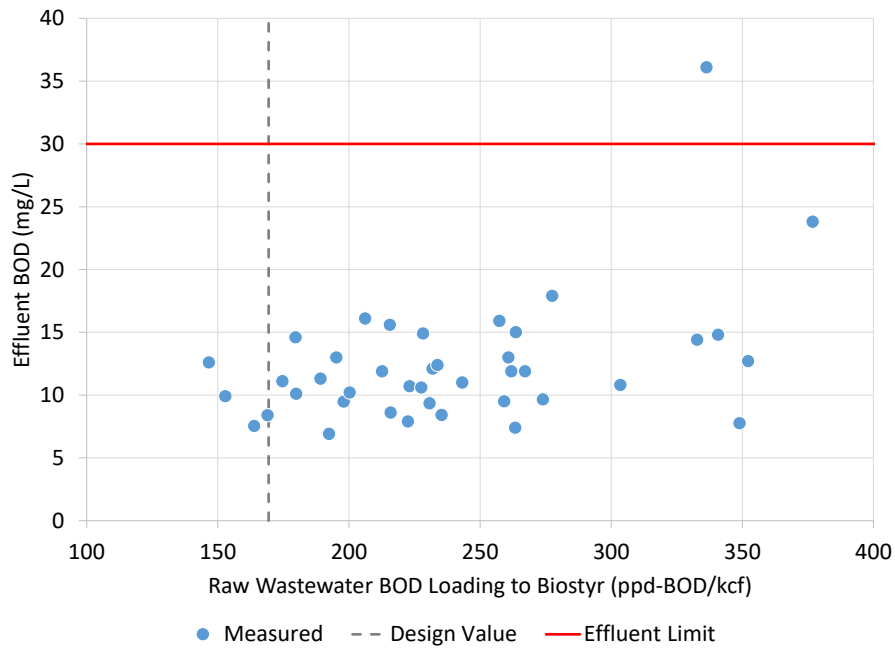


Figure 8. Raw Wastewater BOD Loadings to Biostyr and Effluent Quality

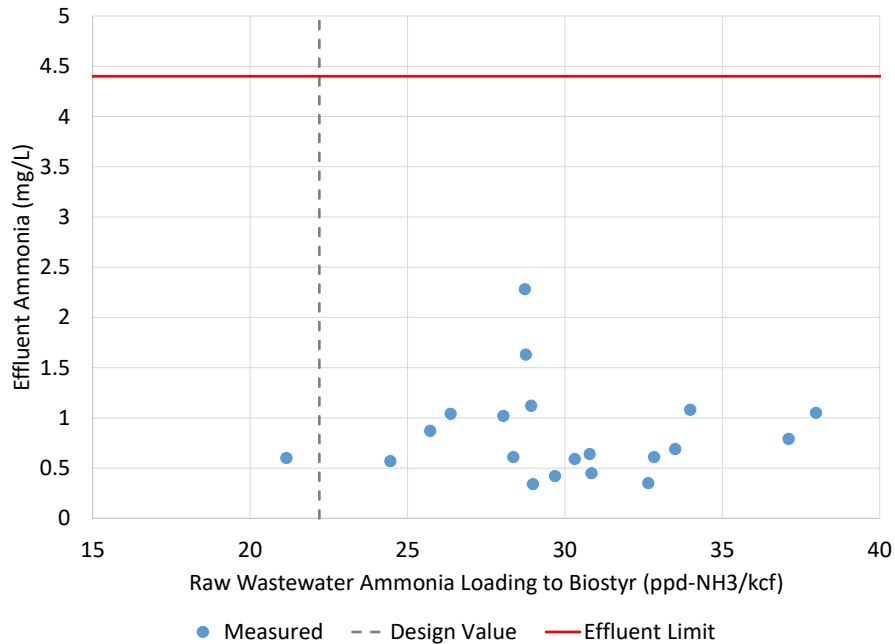


Figure 9. Raw Wastewater NH3 Loadings to Biostyr and Effluent Quality

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Proposed Rated Capacity

Based on the excellent cold-weather BOD removal performance of the Actiflo-Biostyr system, the Heart of the Valley Metropolitan Sewerage District requests the WRRF capacity be increased to an influent BOD loading that corresponds to 250 ppd-BOD/kcf of active Biostyr media. Assuming one of the Biostyr cells will be off line for backwashing, this volumetric loading would increase the BOD capacity from 14,651 ppd-BOD to 18,918 ppd, a 29% increase. **See Figure 10.**

Based on the excellent cold-weather NH3 removal performance of the Actiflo-Biostyr system, the Heart of the Valley Metropolitan Sewerage District requests the WRRF capacity be increased to an influent NH3 loading that corresponds to 30 ppd-BOD/kcf of active Biostyr media. Assuming one of the Biostyr cells will be offline for backwashing, this volumetric loading would increase the NH3 capacity from 1,920 ppd-BOD to 2,270 ppd, an 18% increase. **See Figure 10.**

Active Biostyr Volume		
Parameter	Units	Value
Area per Cell	sf	940
Cell Height	ft	11.5
Number of Cells	#	7
Total Volume	cf	75,670
	kcf	75.67

WRRF Capacity / Maximum Month Condition				
Raw Wastewater			Proposed Biostyr Volumetric Loading (ppd/kcf)	% Increase from Original Design
Parameter	Units	Value		
Flow	mgd	11.9		
BOD	ppd	18,918	250	129%
TSS	ppd	16,278		
NH3	ppd	2,270	30	118%
TP	ppd	407		

Figure 10. Proposed BOD and NH3 Capacity Ratings

Solids Train Analysis

The requested revised capacity increases the maximum month BOD and NH3 loadings by 29% and 18%, respectively. This section confirms that the existing solids processes can accommodate those increased loadings.

*Maximum Month Solids Production and Solids-Handling Capacity Analysis*

The proposed higher maximum month BOD and NH3 loadings to the Actiflo-Biostyr process will produce more biological solids. **Figure 11** shows [1] maximum month Biostyr solids will increase 29%, [2] maximum month total solids production will increase 8%, and all solids-handling processes are adequately sized for these increases.



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Process and Parameter		Original Design Rating	Proposed Rating	Comments
<b>Maximum Month Pre-Digestion Solids</b>				
Actiflo Solids	ppd	17,092	17,092	No Change
Biostyr Solids	ppd	6,593	8,513	129%
Total Solids	ppd	23,685	25,605	108%
Total Solids Concentration	%	0.30%	0.30%	Hydrocyclone underflow
Total Solids	gpd	946,643	1,023,381	108%
<b>Gravity Thickener</b>				
Surface Area	sf	3,600	3,600	
<b>SOR</b>	<b>gpd/sf</b>	<b>263</b>	<b>284</b>	<b>M+E: 380 - 760 gpd/sf OK</b>
Total Thickened Solids Concentration	%	3.00%	3.00%	
Total Solids	ppd	23,685	25,605	
Total Solids	gpd	94,664	102,338	
<b>Dissolved Air Flotation</b>				
Surface Area	sf	1,536	1,536	
<b>Solids Loading</b>	<b>pph/sf</b>	<b>0.64</b>	<b>0.69</b>	<b>M+E: 0.83 - 1.25 pph/sf OK</b>
Total Thickened Solids Concentration	%	5.00%	5.00%	
Total Solids	ppd	23,685	25,605	
Total Solids	gpd	56,799	61,403	
Total Volatile Solids	ppd	17,898	19,626	70% VS Actiflo, 90% VS BAF
<b>ATAD</b>				
Volume	Mgal	0.639	0.639	
<b>HRT</b>	<b>d</b>	<b>11</b>	<b>10</b>	<b>M+E: 6 - 8 d OK</b>
<b>VS Loading</b>	<b>ppd/kcf</b>	<b>210</b>	<b>230</b>	<b>M+E: 200 - 260 ppd/kcf OK</b>
<b>Maximum Month Post-Digestion Solids</b>				
VS Destruction in ATAD	%	65%	65%	
Total Solids	ppd	12,051	12,848	107%

Figure 11. Maximum Month Solids Capacity Analysis

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*Annual Average Biosolids Production and Storage Analysis*

The proposed higher BOD and NH3 loadings to the Actiflo-Biostyr process will produce more biological solids and, in turn, might increase annual average biosolids quantities. Historical biosolids production values and concentrations are shown in **Figure 12**. The historical weighted average concentration of land applied biosolids is 7.36%.

Year	Land Applied Volume (gallons)	Land Applied Mass (1000 kg)	Land Applied Mass (lbs)	Solids Concentration (%)
2019	1,118,751	217	478,485	5.13%
2020	4,529,804	1,425	3,142,125	8.32%
2021	3,185,717	743	1,638,315	6.17%
Weighted Average				7.36%

Figure 12. Historical Average Biosolids Quantities and Concentrations

Figure 13 shows the existing biosolids storage volume is adequate assuming, conservatively, that annual average biosolids quantities will increase at the same rate as the maximum month quantities.

Process and Parameter	Original Design Rating	Proposed Rating	Comments
Average Month Post-Digestion Solids			
Total Solids	ppd	9,780	10,427 107%
Total Solids Concentration	%	7.36%	7.36% Historical data
Total Solids	gpd	15,933	16,986
180-Day Volume Required	Mgal	2.87	3.06 Actual = 4.2 Mgal OK

Figure 13. Annual Average Biosolids Production Quantities and Storage Requirement

Summary

The preceding analysis demonstrates the existing WRRF serving the HOVMSD possess adequate liquid and solids train capacities to produce permit-compliant effluent and biosolids if the Department increases its maximum month BOD and NH3 capacities to 18,918 ppd and 2,270 ppd, respectively. The HOVMSD requests the Department of Natural Resources to adjust the rated-capacity accordingly. The Figures that follow show how recent historical flows and loadings compare to current and requested (BOD and NH3) capacities.

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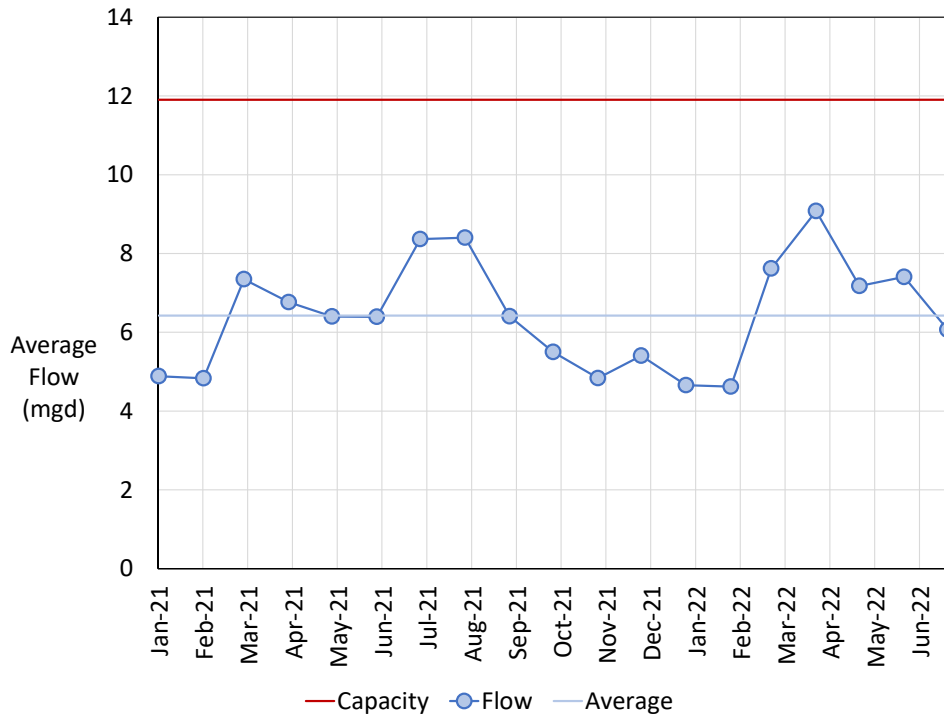


Figure 14. Rated Flow Capacity and Recent Historical Monthly Average Flows

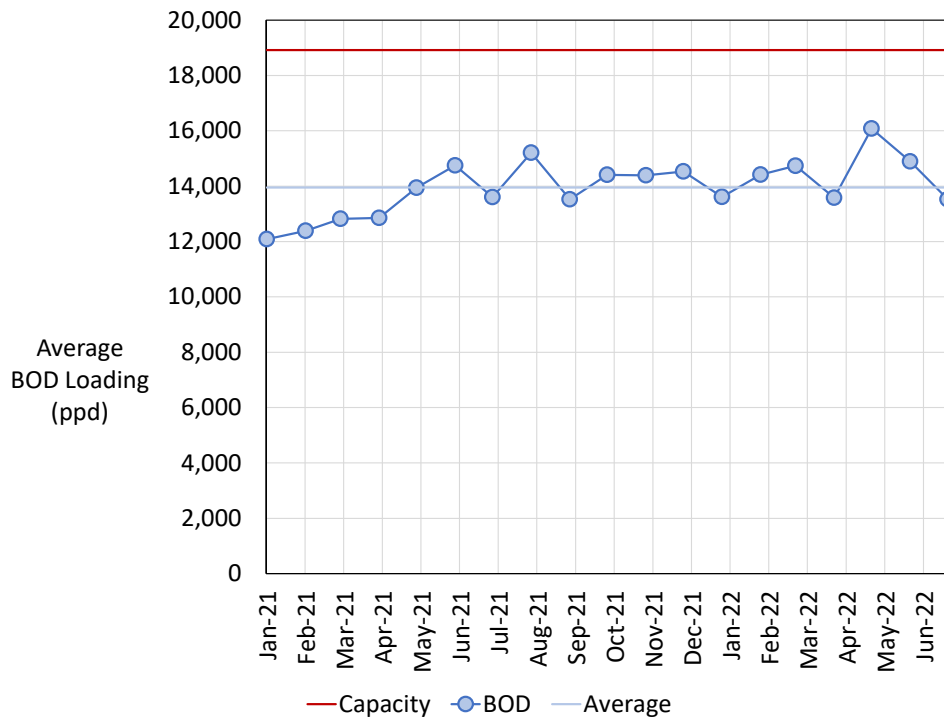


Figure 15. Requested BOD Capacity and Recent Historical Monthly Average Loadings

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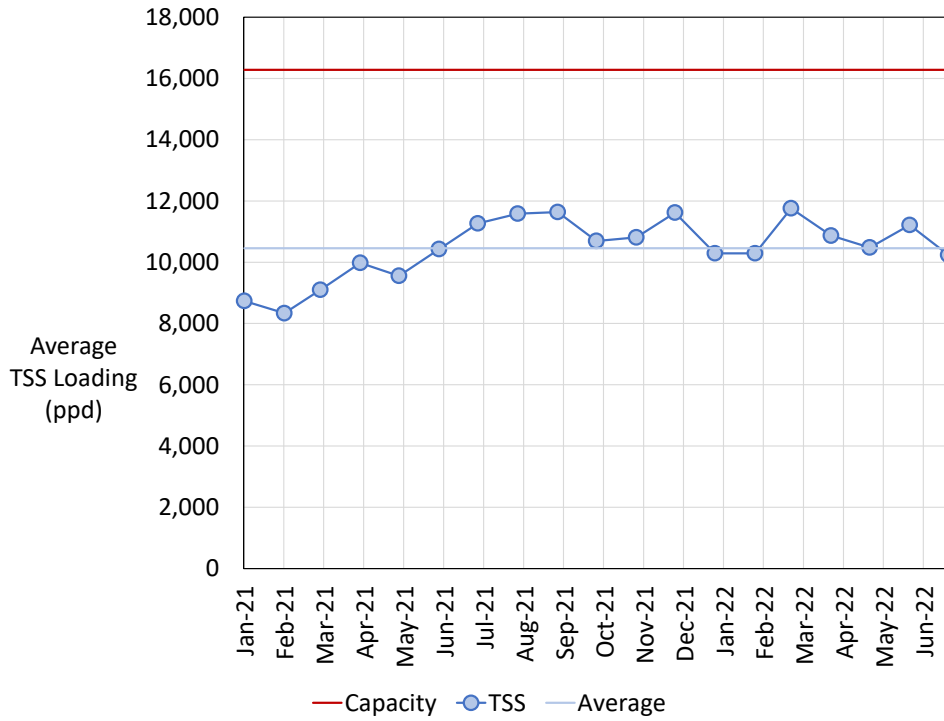


Figure 15. Rated TSS Capacity and Recent Historical Monthly Average Loadings

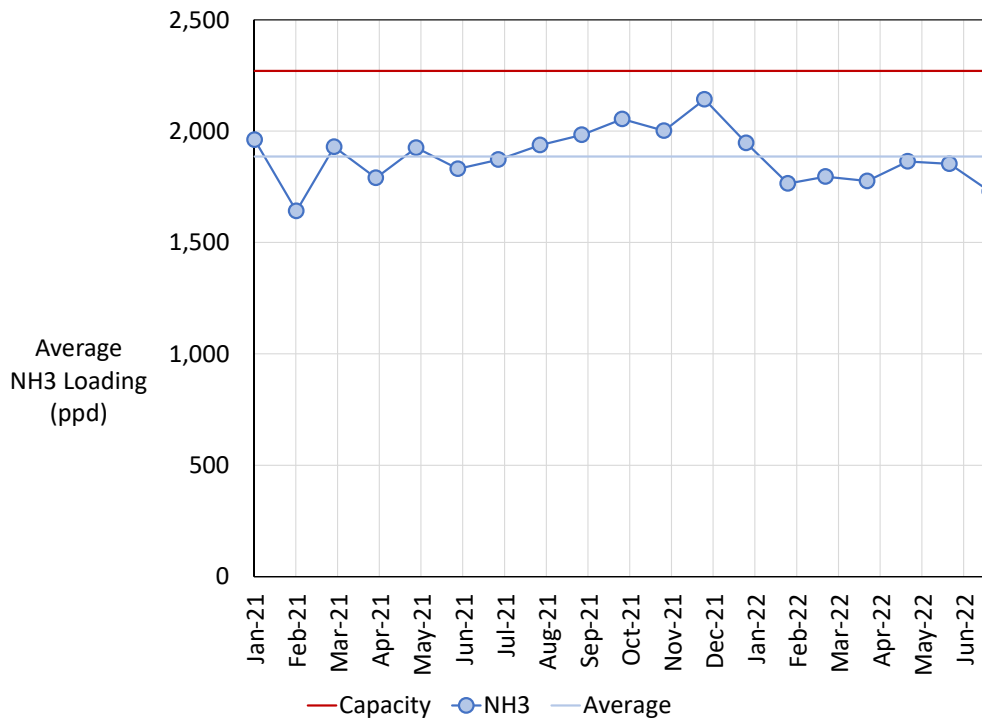


Figure 15. Requested NH3 Capacity and Recent Historical Monthly Average Loadings

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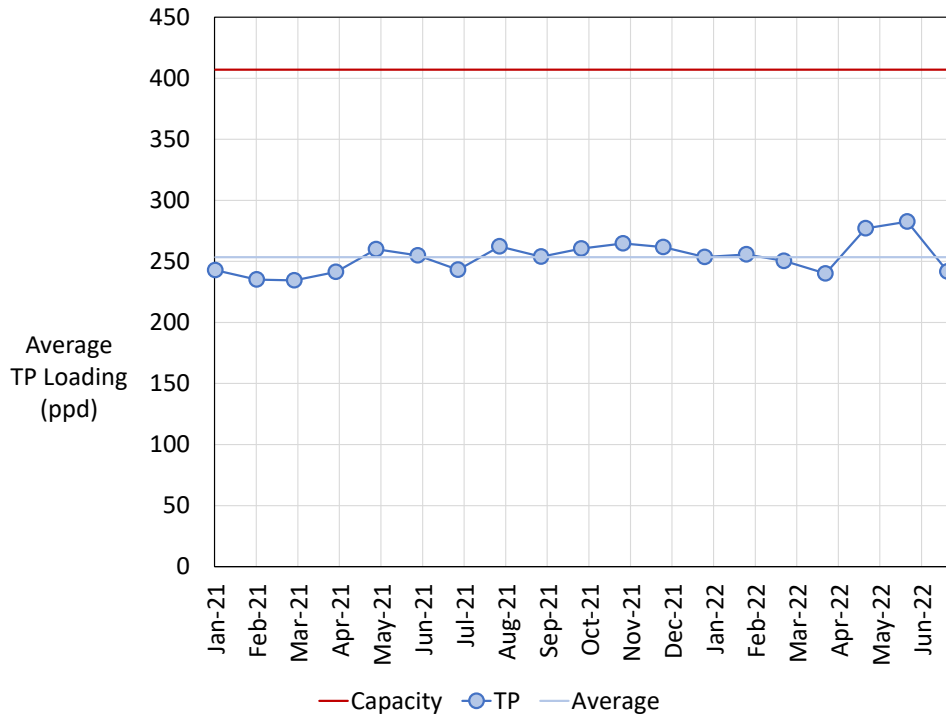


Figure 15. Rated TP Capacity and Recent Historical Monthly Average Loadings

# Appendix A

Original Design Basis

### III. DESIGN CRITERIA

Parameter Design Year	Design Parameter														
DESIGN YEAR POPULATION	<b>2028</b>														
DESIGN YEAR FLOW, mgd	67,827														
<ul style="list-style-type: none"> <li>■ Average</li> <li>■ Maximum 30-Day (1.4:1)</li> <li>■ Normal Maximum Day (3.1:1)</li> <li>■ Maximum day, 100-year storm</li> <li>■ Peak Hour, 100-year storm</li> <li>■ Average 5-Day/Week Flow</li> <li>■ Average Weekend Flow</li> <li>■ Peak Daily Diurnal</li> <li>■ Average Dry Weather Flow</li> <li>■ Minimum Flow</li> </ul>	8.5 11.9 26.4 35 60 9.2 7.7 13.7 5.7 3.9														
RECYCLE FLOWS <ul style="list-style-type: none"> <li>■ Actiflo Underflow To New Gravity Thickener, Overflow To Peak Flow Pump Station</li> </ul>	<table border="0"> <thead> <tr> <th style="text-align: left;"><u>Forward Flow</u></th> <th style="text-align: left;"><u>Recycle Flow</u></th> </tr> </thead> <tbody> <tr> <td>0 to 15 mgd</td> <td>352 gpm, 0.127 mgd</td> </tr> <tr> <td>15 to 20 mgd</td> <td>704 gpm, 1 mgd</td> </tr> <tr> <td>20 to 30 mgd</td> <td>1,056 gpm, 1.5 mgd</td> </tr> <tr> <td>30 to 40 mgd</td> <td>1,408 gpm, 2 mgd</td> </tr> <tr> <td>40 to 50 mgd</td> <td>1,760 gpm, 2.5 mgd</td> </tr> <tr> <td>50 to 60 mgd</td> <td>2,112 gpm, 3 mgd</td> </tr> </tbody> </table>	<u>Forward Flow</u>	<u>Recycle Flow</u>	0 to 15 mgd	352 gpm, 0.127 mgd	15 to 20 mgd	704 gpm, 1 mgd	20 to 30 mgd	1,056 gpm, 1.5 mgd	30 to 40 mgd	1,408 gpm, 2 mgd	40 to 50 mgd	1,760 gpm, 2.5 mgd	50 to 60 mgd	2,112 gpm, 3 mgd
<u>Forward Flow</u>	<u>Recycle Flow</u>														
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40 to 50 mgd	1,760 gpm, 2.5 mgd														
50 to 60 mgd	2,112 gpm, 3 mgd														
<ul style="list-style-type: none"> <li>■ BAF Backwash Pumped To Actiflo               <ul style="list-style-type: none"> <li>◆ Average (backwash every 48-hours)</li> <li>◆ Maximum (backwash every 24-hours)</li> </ul> </li> <li>■ DAF Subnatant to Headworks               <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum 30-Day</li> </ul> </li> <li>■ Sludge Storage Decant To Peak Flow Wet Well               <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum 30-Day</li> </ul> </li> </ul>	<table border="0"> <tbody> <tr> <td>0.80 mgd</td> <td><i>8 x .8 mgd</i></td> </tr> <tr> <td>1,683 gpm, 1.6 mgd</td> <td></td> </tr> <tr> <td>28,053 gpd</td> <td><i>19.5 gpm</i></td> </tr> <tr> <td>33,665 gpd</td> <td></td> </tr> <tr> <td>21,904 gpd</td> <td><i>15.2 gpm</i></td> </tr> <tr> <td>26,285 gpd</td> <td></td> </tr> </tbody> </table>	0.80 mgd	<i>8 x .8 mgd</i>	1,683 gpm, 1.6 mgd		28,053 gpd	<i>19.5 gpm</i>	33,665 gpd		21,904 gpd	<i>15.2 gpm</i>	26,285 gpd			
0.80 mgd	<i>8 x .8 mgd</i>														
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28,053 gpd	<i>19.5 gpm</i>														
33,665 gpd															
21,904 gpd	<i>15.2 gpm</i>														
26,285 gpd															
BOD, lbs./day															
<ul style="list-style-type: none"> <li>■ Average</li> <li>■ Maximum 30-Day (1.2:1)</li> <li>■ Maximum Day (2.4:1)</li> </ul>	12,209 14,651 29,302														
TSS, lbs./day															
<ul style="list-style-type: none"> <li>■ Average</li> <li>■ Maximum 30-Day (1.2:1)</li> <li>■ Maximum Day (2.4:1)</li> </ul>	13,565 16,278 32,556														
PHOSPHORUS															
<ul style="list-style-type: none"> <li>■ Average</li> <li>■ Maximum 30-Day (1.2:1)</li> <li>■ Maximum Day (3:1)</li> </ul>	339 407 1,017														
AMMONIA N, lbs./day (based upon 16 mg/l)															
<ul style="list-style-type: none"> <li>■ Average</li> <li>■ Maximum 30-Day (1.2:1)</li> <li>■ Maximum Day (2.5:1)</li> </ul>	1,600 1,920 3,005														

Parameter Design Year	Design Parameter 2028
<b>HAULED WASTE RECEIVING STATION</b> (Conversion Of Existing Gravity Thickener) <ul style="list-style-type: none"> <li>■ Number Of Units</li> <li>■ Volume @ High Water Level</li> <li>■ Number Of Pumps</li> <li>■ Type               <ul style="list-style-type: none"> <li>◆ Capacity Each Pump, gpm</li> <li>◆ TDH, each</li> <li>◆ Horsepower, each</li> </ul> </li> </ul>	1 49,800 gallons 2 Submersible Grinder 56 32 2.7
<b>INFLUENT FLOW MONITORING</b> <ul style="list-style-type: none"> <li>■ Number</li> <li>■ Type</li> <li>■ Size</li> </ul>	Parshall Flume 2 (1 normal flow, 1 peak flow) 36-inch
<b>FINE SCREENS</b> <ul style="list-style-type: none"> <li>■ Normal Flow               <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Type</li> <li>◆ Opening Size, mm</li> <li>◆ Horsepower, screens</li> <li>◆ Horsepower, washer compactor</li> </ul> </li> <li>■ Peak Flow               <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Type</li> <li>◆ Opening Size, mm</li> <li>◆ Horsepower, screen</li> <li>◆ Horsepower, washer compactor</li> </ul> </li> </ul>	3 (2 screens, 1 washer compactor) Mechanical Step Screens 10 3 2.3  2 (1 screen, 1 washer compactor) Mechanical Rake Bar Screens 6 2 5
<b>ACTIFLO INFLUENT PUMP STATION</b> <ul style="list-style-type: none"> <li>■ To Actiflo Firm Capacity Of 25 mgd               <ul style="list-style-type: none"> <li>◆ Number Of Pumps</li> <li>◆ Type</li> <li>◆ Capacity, Each, gpm</li> <li>◆ TDH, feet</li> <li>◆ Horsepower, each</li> <li>◆ VFD's</li> </ul> </li> </ul>	3 Vertical Turbine Solids Handling 10,000 / 8,700 / 4,200 49/ 48 / 44 150 Yes
<b>MODIFIED PE PUMPS - PEAK FLOW PUMP STATION</b> <ul style="list-style-type: none"> <li>■ To Actiflo &amp; Disinfection               <ul style="list-style-type: none"> <li>◆ Number Of Pumps</li> <li>◆ Type</li> <li>◆ Capacity, Each, gpm</li> <li>◆ TDH, feet</li> <li>◆ Horsepower, each</li> <li>◆ VFD's</li> </ul> </li> </ul>	4 + 1-future Vertical Intermediate Shaft Centrifuge Pump 8,133 59 150 Yes
<b>NORMAL RECYCLE FLOWS - PROCESS RETURN FLOW PUMPS</b> <ul style="list-style-type: none"> <li>■ To Actiflo (former RAS Pumps)               <ul style="list-style-type: none"> <li>◆ Number Of Pumps</li> <li>◆ Type</li> <li>◆ Capacity, each, gpm (@ pump start)</li> <li>◆ TDH, feet</li> <li>◆ Horsepower, each</li> <li>◆ VFD's</li> </ul> </li> </ul>	3 Vertical Dry Pit Centrifuge 675 43 25 Yes



Parameter Design Year	Design Parameter 2028
<b>GRIT REMOVAL</b> ■ Number Of Units ■ Type ■ Capacity, each unit, mgd ■ Horsepower	2 Vortex 30 2
<b>GRIT WASHER</b> ■ Type ■ Horsepower, stirrer ■ Horsepower, screw conveyor	Coanda 1 1.5
<b>ACTIFLO HIGH RATE CLARIFICATION SYSTEM</b> ■ Total Design Flow, mgd ■ Peak Hydraulic Capacity, mgd ■ Number Of Trains ■ Capacity Per Train, mgd	60 70 2 30
<b>COAGULATION TANK DESIGN (@ 30 mgd per train)</b> ■ HRT, minute ■ Number Of Tanks Per Train ■ Length, feet ■ Width, feet ■ Sidewater Depth, feet = <i>2,154.2 gallons</i> ■ Mixer ◆ rpm ◆ Horsepower ◆ VFD's	1 1 12 12 20 37 10 No
<b>INJECTION TANK DESIGN (@ 30 mgd per train)</b> ■ HRT, minute ■ Number Of Tanks Per Train ■ Length, feet ■ Width, feet ■ Sidewater Depth, feet ■ Mixer ◆ rpm ◆ Horsepower ◆ VFD's	1 1 12 12 20 37 Max. 10 Yes
<b>MATURATION TANK DESIGN (@ 30 mgd per train)</b> ■ HRT, minute ■ Number Of Tanks Per Train ■ Length, feet ■ Width, feet ■ Sidewater Depth, feet ■ Mixer ◆ rpm ◆ Horsepower ◆ VFD's	3 1 18.1 25.3 20 20 Max. 15 Yes

Parameter Design Year	Design Parameter 2028
<b>SETTLING TANK DESIGN</b>	
■ Number Of Tanks Per Train	1
■ Length, feet	25.3
■ Width, feet	25.3
■ Sidewater Depth, feet	20
■ Plan View Settling Area, sq.ft.	417
■ Overflow Rate @ 30 mgd Per Train, gpm/sq.ft.	50
■ Overflow Rate @ 35 mgd Per Train, gpm/sq.ft.	58.3
■ Scraper Mechanism	
◆ rpm	0.25 Max.
◆ Horsepower	5
◆ VFD's	Yes
<b>SAND RECIRCULATION CIRCUIT DESIGN</b>	
■ Number Of Pumps Per Train	2 duty + 1 standby
■ Type	Centrifugal Slurry Pumps
■ Total Dynamic Head, psi	45
■ Pump Capacity, gpm	440
■ Horsepower	25
■ VFD's	No
■ Number of Hydrocyclones Per Pump	1
■ Capacity, gpm	440
■ Estimated Sludge Concentration, % solids	0.1 to 0.5
■ Sludge Discharge Per Train @ Design Flow, gpm	704
■ Capacity Each Train, wo/sand @ 13 gpm/sq.ft., mgd	7.8
■ Capacity Each Train, w/sand @ 70 gpm/sq.ft., mgd	42
<b>BALLASTED SEDIMENTATION PERFORMANCE</b>	
■ CBOD Removal	50%
■ TSS Removal	70%
■ NH3N Removal	0%
■ Phosphorus Removal	75%
■ Chemical Used	Fe <sub>2</sub> (SO <sub>4</sub> ) <sub>3</sub>
■ Chemical Dose	50 to 150 mg/l
■ Polymer Dose	1 to 2.5 mg/l

Parameter Design Year	Design Parameter 2028
<b>POLYMER CHEMICAL FEED SYSTEM (continued)</b>	
<ul style="list-style-type: none"> <li>■ Polymer Metering Pumps <ul style="list-style-type: none"> <li>◆ Number Of Pumps Per Train</li> <li>◆ Type</li> <li>◆ Capacity Each, gpd</li> <li>◆ Horsepower</li> </ul> </li> </ul>	2 + 1 standby Progressive Cavity 1,440 to 28,800 2
<b>LOADINGS TO BIOSTYR BIOLOGICAL AERATED FILTER</b>	
<ul style="list-style-type: none"> <li>■ Flow <ul style="list-style-type: none"> <li>◆ Maximum Daily Flow, mgd</li> </ul> </li> <li>■ CBOD, lbs./day <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum, 30-day</li> <li>◆ Maximum Day</li> </ul> </li> <li>■ CBOD, mg/l <ul style="list-style-type: none"> <li>◆ Average (8.5 mgd)</li> <li>◆ Maximum, 30-day (11.9 mgd)</li> <li>◆ Maximum Day (26.4 mgd)</li> <li>◆ @ Dry Weather Flow (5.7 mgd)</li> </ul> </li> <li>■ TSS, lbs./day <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum, 30-day</li> <li>◆ Maximum Day</li> </ul> </li> <li>■ TSS, mg/l <ul style="list-style-type: none"> <li>◆ Average (8.5 mgd)</li> <li>◆ Maximum, 30-day (11.9 mgd)</li> <li>◆ Maximum Day (26.4 mgd)</li> <li>◆ @ Dry Weather Flow (5.7 mgd)</li> </ul> </li> <li>■ Ammonia, lbs./day <sup>1</sup> <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum, 30-day</li> <li>◆ Maximum Day</li> <li>◆ Peak Daily Diurnal Ammonia Load, lbs./day</li> <li>◆ @ 13.9 mgd and 32 mg/l NH<sub>3</sub>N</li> </ul> </li> </ul>	26.4  6,105 7,326 14,651  86 mg/l 74 mg/l 67 mg/l 128 mg/l  4,070 4,883 9,767  57 mg/l 49 mg/l 44 mg/l 86 mg/l  1,720 (24.3 mg/l) 2,100 (29.6 mg/l) 3,306 3,710
<sup>1</sup> Ammonia loadings include 10% safety factor due to recycle flows and hauled in wastes.	
<ul style="list-style-type: none"> <li>■ Ammonia, mg/l <ul style="list-style-type: none"> <li>◆ Average (8.5 mgd)</li> <li>◆ Maximum, 30-day (8.5 mgd)</li> <li>◆ Maximum Day (26.4 mgd)</li> <li>◆ @ Average Dry Weather (5.7 mgd)</li> </ul> </li> </ul>	24.3 mg/l 29.6 mg/l 15 mg/l 36 mg/l

Parameter Design Year	Design Parameter 2028
<b>LOADINGS TO BIOSTYR BIOLOGICAL AERATED FILTER (cont.)</b>	
<ul style="list-style-type: none"> <li>■ Effluent Requirements, mg/l, monthly average <ul style="list-style-type: none"> <li>◆ CBOD, average</li> <li>◆ CBOD, maximum month</li> <li>◆ TSS, average</li> <li>◆ TSS, maximum month</li> <li>◆ NH3N, Average June-September</li> <li>◆ NH3N, Average January-March</li> <li>◆ NH3N, Average April-May</li> <li>◆ NH3N, Average October-December</li> <li>◆ NH3N, daily maximum</li> <li>◆ Phosphorus, average</li> </ul> </li> <li>■ Minimum Waste Temperature</li> <li>■ Flow, mgd, maximum day</li> </ul>	<ul style="list-style-type: none"> <li>15 mg/l</li> <li>25 mg/l</li> <li>20 mg/l</li> <li>25 mg/l</li> <li>3.6 mg/l</li> <li>10 mg/l</li> <li>11 mg/l</li> <li>18 mg/l</li> <li>20 mg/l</li> <li>1 mg/l</li> <li>7°C</li> <li>26</li> </ul>
<b>BIOSTYR SYSTEM</b>	
<ul style="list-style-type: none"> <li>■ Number Of Filters</li> <li>■ Size, each cell, sq.ft.</li> <li>■ Media Size, mm</li> <li>■ Total Filter Area, sq.ft.</li> <li>■ Filtration Rate, maximum, gpm/sq.ft.</li> <li>■ BAF Backwash, mgd</li> <li>■ Height Of Media, feet</li> <li>■ Process Air/Cell, scfm <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum Month</li> <li>◆ Peak Day</li> </ul> </li> <li>■ Backwash Air/Cell, scfm</li> <li>■ Average Air Requirement, scfm</li> <li>■ Maximum Month Air Requirement, scfm</li> <li>■ Peak Day Air Demand, scfm</li> <li>■ Blowers <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Size, each unit, scfm</li> <li>◆ Discharge Pressure</li> <li>◆ Horsepower, each</li> </ul> </li> <li>■ Backwashing <ul style="list-style-type: none"> <li>◆ Frequency</li> <li>◆ Backwash Volume/Cell, gpd</li> <li>◆ Backwash Volume, Daily, mgd</li> </ul> </li> <li>■ Spent Backwash Tanks <ul style="list-style-type: none"> <li>◆ Number Of Tanks (3 existing Unox tanks)</li> <li>◆ Total Volume @ Water Surface 611.78, gallons</li> </ul> </li> <li>■ Spent Backwash Return Pumps <ul style="list-style-type: none"> <li>◆ Number</li> <li>◆ Type</li> <li>◆ Capacity, each, gpm</li> <li>◆ TDM, feet</li> <li>◆ Horsepower</li> </ul> </li> </ul>	<ul style="list-style-type: none"> <li>8</li> <li>940</li> <li>4.0</li> <li>7,520</li> <li>3.08</li> <li>1</li> <li>11.5</li> <li>258</li> <li>310</li> <li>735</li> <li>735</li> <li>2,800 (w/backwash air)</li> <li>3,215 (w/backwash air)</li> <li>6,615</li> <li>8 + 1 Standby</li> <li>726</li> <li>14.04 psig</li> <li>75</li> <li>1/day/cell</li> <li>202,000</li> <li>1.62</li> <li>3</li> <li>250,000</li> <li>2</li> <li>Dry Pit Submersible</li> <li>1,683</li> <li>51.2</li> <li>40</li> </ul>

Parameter Design Year	Design Parameter
<b>DISINFECTION</b>	
■ Existing Chlorine Contact Tanks	
◆ Number Of Rectangular Tanks	1
◆ Volume, gal	334,955
◆ HRT @ 8.5 mgd, mins.	57
■ Secondary Clarifier Tanks Converted To Peak Flow Chlorine Tank	
◆ Number Of Tanks	2
◆ Volume, two tanks, gal	646,272
◆ HRT @ 35 mgd, mins.	26.6
■ Chlorination System	
◆ Chlorine Demand, mg/l	1.5
◆ Chlorine Demand, lbs./day	
- @ 8.5 mgd	107
- @ 11.9 mgd	149
◆ @ 60 mgd	757
◆ Total Hypochlorite Demand, gpd	
- @ 8.5 mgd	107
- @ 11.9 mgd	149
- @ 60 mgd	757
◆ Hypochlorite Demand @ Existing Contact Tanks	
- @ 8.5 mgd, gpd	214
- @ 11.9 mgd, gpd	298
- @ 26.4 mgd, gpd	672
◆ Peak Hypo Demand @ Each Peak Contact Tank, gpd	552
■ Hypochlorite Storage Tanks	
◆ Number Of Tanks	1
◆ Volume, each, gal	12,000
◆ HRT @ 50% Dilution, days/tank	
- @ 8.5 mgd	56
- @ 11.9 mgd	40
■ Hypochlorite Transfer Pump	
◆ Number Of Units	1
◆ Type	Air Diaphragm
◆ Capacity, gpm	49 max.
■ Existing Cl <sub>2</sub> Contact Tank	
◆ Hypochlorite Feed Pumps	
- Number Of Pumps	2 + 1 spare
- Type	Diaphragm Metering
- Capacity, each, gpd	29 - 576
- Horsepower	½
■ Peak Flow Cl <sub>2</sub> Contact, each tank	
◆ Number Of Pumps	1
◆ Type	Diaphragm Metering
◆ Capacity, each, gpd	29 - 576
◆ Horsepower	½
■ Sodium Bisulfite	
◆ Equivalent Dose To SO <sub>2</sub>	
. lbs. Bisulfite/gal @ 38% Solution	1.5
- gpd of Bisulfite Needed	3.17
@ 8.6 mgd	
@ 26.4 mgd	34
@ 60 mgd	10636

Parameter Design Year	Design Parameter	
	2028	
<b>DISINFECTION (continued)</b>		
■ Sodium Bisulfite Storage Tank		
◆ Number Of Tanks	1	
◆ Volume Of Tank, gallons	2,000	
◆ HRT, days		
- @ 8.5 mgd	59	
- @ 11.9 mgd	49	
◆ Sodium Bisulfite Feed Pumps, gpd		
- Type	10.8 to 216	
- 2 Pumps @	Diaphragm Metering	
- 1 Pump @	6 to 120	
- Horsepower	1/2	
<b>SLUDGE PRODUCTION</b>		
■ Primary Sludge	<u>Start-Up</u>	<u>2028</u>
◆ BOD Removal, %	50	50
◆ TSS Removal, %	70	70
◆ Phosphorus Removal, %	75	75
◆ NH3N Removal, %	0	0
■ Total Suspended Solids Removed, lbs./day		
◆ Average	7,093	9,496
◆ Maximum, 30-day	8,546	11,395
◆ Maximum Day (x 2.4)	17,092	22,789
■ Total Actiflo Sludge Production, lbs./lb. TSS Removed <sup>(1)</sup>		
◆ Fe2(SO4)3 Dose of 90 mg/l	1.6	
◆ Fe2(SO4)3 Dose of 60 mg/l	1.5	
◆ Fe2(SO4)3 Dose Of 50 mg/l	1.4	
<sup>1</sup> Based upon mass balance during pilot testing.		
■ Total Actiflo Sludge, lbs./day @ 60 mg/l Dose		
◆ Average	10,640	14,244
◆ Maximum Month	12,820	17,093
◆ Maximum Day	25,638	34,184
■ % Volatiles - Actiflo Sludge	70	70
■ Volatiles, lbs./day		
◆ Average	7,448	9,971
◆ Maximum Month	8,974	11,965
◆ Maximum Day	17,947	23,929
■ BAF Sludge, lbs./day to Actiflo		
◆ Yield, lbs./lb. CBOD Removed	0.9	0.9
◆ Average	3,915	5,495
◆ Maximum, 30-day (x 1.2)	4,698	6,593
◆ Maximum Day (x 2.5)	9,788	13,186
■ Total Sludge, lbs./day [@ 60 mg/l Fe <sub>2</sub> (SO <sub>4</sub> ) <sub>3</sub> ]		
◆ Average	14,555	19,739
◆ Maximum, 30-day	17,518	23,686
◆ Maximum Day	35,426	47,350
■ Volatiles, %		
◆ Actiflo Sludge	70%	70%
◆ BAF Sludge	90%	90%
■ BAF Volatile Sludge, lbs./day		
◆ Average	3,524	5,351
◆ Maximum Month	4,228	5,934
◆ Maximum Day	8,809	11,867

Parameter Design Year	Design Parameter	
	2028	
	Start-Up	2028
<b>SLUDGE PRODUCTION (continued)</b>		
■ Total Volatiles, lbs./day		
◆ Average	10,972	15,322
◆ Maximum 30-day	13,202	17,899
◆ Maximum Day	26,756	35,796
■ ATAD Performance VSS Destruction, %	65	65
■ VSS Destroyed, lbs./day		
◆ Average	7,132	9,959
◆ Maximum, 30-day	8,581	11,634
◆ Maximum Day	17,391	23,267
■ Sludge To Storage, lbs./day		
◆ Average	7,423	9,780
◆ Maximum, 30-day	8,937	12,052
◆ Maximum Day	18,035	24,083
■ Sludge From Storage, gpd @ 5%		
◆ Average	17,801	23,453
■ Sludge Storage Decant, gpd	17,103	23,883
■ Ammonia Concentration, mg/l	200	200
■ Ammonia Sidestream, lbs./day	29	40
<b>SLUDGE GRAVITY THICKENER</b>		
■ Number Of Units	1 (existing Primary Clarifier 4) + 1 spare (Primary Clarifier 3)	
■ Length, feet	60	
■ Width, feet	60	
■ Sidewater Depth, feet	10	
■ SOR, gpd/sq.ft.		
◆ Maximum @ 4.4 mgd	1,222	
■ Solids Load, lbs./sq.ft.		
◆ Average	5.1	
◆ Maximum Month	6.1	
■ % Solids To DAF	3	
■ Volume To Sludge To DAF, gpd		
◆ Average	78,893	
◆ Maximum Month	94,668	
◆ Maximum Day	189,249	
<b>THICKENED SLUDGE TRANSFER PUMPS</b>		
■ Number Of Units	1 new & 2 existing	
■ Type (new)	Rotary lobe	
■ Capacity (new), gpm	250	
■ Total Dynamic Head (new), psig	20	
■ Horsepower (new)	7.5	
■ VFD	Yes	
<b>ACTIFLO SCUM PUMPS</b>		
■ Number Of Units	2	
■ Type	Vertical Pedestal Chopper	
■ Capacity, gpm	310	
■ Total Dynamic Head, feet	73	
■ Horsepower	5	
■ VFD	No	

Parameter Design Year	Design Parameter 2028
<b>DISSOLVED AIR FLOTATION THICKENERS (existing)</b> <ul style="list-style-type: none"> <li>■ Number Of Units</li> <li>■ Dimensions, each unit, feet <ul style="list-style-type: none"> <li>◆ Length</li> <li>◆ Width</li> <li>◆ Surface Area, sq.ft., each</li> </ul> </li> <li>■ Surface Area, sq.ft., total</li> <li>■ Solids Loading/Day, lbs./day/sq.ft. <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum Month</li> <li>◆ Maximum Day</li> </ul> </li> <li>■ Volume From DAF @ 5.0%, gpd <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum Month</li> </ul> </li> </ul>	2  48 16 768 1,536  12.9 15.4 30.8  47,333 56,801
<b>THICKENED SLUDGE DAY TANK</b> <ul style="list-style-type: none"> <li>■ Number Of Units</li> <li>■ Area Above Feet, sq.ft.</li> <li>■ Maximum Sidewater Depth, feet</li> <li>■ Maximum Volume, gallons</li> <li>■ Mixer <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Type</li> <li>◆ Capacity</li> <li>◆ Horsepower</li> <li>◆ VFD</li> </ul> </li> </ul>	1 530.9 27 95,000  1 Submersible 11,729 15.4 No
<b>ATAD FEED PUMPS</b> <ul style="list-style-type: none"> <li>■ Number Of Units</li> <li>■ Type</li> <li>■ Capacity, each, gpm</li> <li>■ Total Dynamic Head, psig</li> <li>■ Horsepower</li> <li>■ VFD's</li> </ul>	2 Rotary Lobe 322 40 20 Yes
<b>ATAD</b> <ul style="list-style-type: none"> <li>■ Tank Diameter, feet</li> <li>■ Total Tank Volume, gal</li> <li>■ Volume Used, gal</li> <li>■ HRT, days <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum Month</li> </ul> </li> <li>■ ATAD Recirculation Pumps <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Type</li> <li>◆ Horsepower, each</li> <li>◆ Capacity, gpm (each)</li> <li>◆ Total Design Head, feet</li> <li>◆ VFD's</li> </ul> </li> </ul>	65 721,894 638,700  13.5 11.2  2 Centrifugal Open Impeller 125 11,000 31 Yes



Parameter Design Year	Design Parameter 2028
ATAD (continued) <ul style="list-style-type: none"> <li>■ ATAD Foam Suppression / Transfer Pumps               <ul style="list-style-type: none"> <li>◆ Number Of Pumps</li> <li>◆ Type</li> <li>◆ Capacity, each, gpd</li> <li>◆ Total Design Head, feet</li> <li>◆ Horsepower</li> <li>◆ VFD's</li> </ul> </li> <li>■ ATAD Blowers               <ul style="list-style-type: none"> <li>◆ Number Of Blowers</li> <li>◆ Capacity, each, gpd</li> <li>◆ Total Design Head, feet</li> <li>◆ Horsepower</li> </ul> </li> </ul>	2 Centrifugal Open Impeller 1,500 85 40  2 + 1 standby 1,175 10.75 100
POST-ATAD NITRIFICATION <ul style="list-style-type: none"> <li>■ Existing Digestion Converted To Post-ATAD Tanks               <ul style="list-style-type: none"> <li>◆ Number Of Tanks</li> <li>◆ Tank Diameter, feet</li> <li>◆ Volume, each, gallons</li> </ul> </li> <li>■ Total Post-ATAD HRT, days               <ul style="list-style-type: none"> <li>◆ Average</li> <li>◆ Maximum Month</li> </ul> </li> <li>■ Post ATAD Recirculation Pumps               <ul style="list-style-type: none"> <li>◆ Number Of Pumps</li> <li>◆ Type</li> <li>◆ Capacity, each, gpm</li> <li>◆ Total Design Head, feet</li> <li>◆ Horsepower</li> <li>◆ VFD's</li> </ul> </li> <li>■ Post ATAD Blowers               <ul style="list-style-type: none"> <li>◆ Number Of Blowers</li> <li>◆ Capacity, each, scfm</li> <li>◆ Design Pressure, psig</li> <li>◆ Horsepower</li> <li>◆ VFD's</li> </ul> </li> </ul>	2 40 200,000  8.5 7.0  2 Centrifugal Open Impeller 4,500 22.5 40 Yes  2 + 1 spare 400 8.15 25 Yes
ATAD ODOR CONTROL BIOFILTER <ul style="list-style-type: none"> <li>■ Biofilter               <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Media Type</li> <li>◆ Media Area, sq.ft.</li> <li>◆ Lava Rock Depth, feet</li> <li>◆ Wood Chip Depth, feet</li> </ul> </li> <li>■ Biofilter Ammonia Scrubber               <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Diameter, feet</li> <li>◆ Height, feet</li> </ul> </li> <li>■ Biofilter Fan               <ul style="list-style-type: none"> <li>◆ Number Of Units</li> <li>◆ Capacity, acfm</li> <li>◆ Design Pressure, inches, w.c.</li> <li>◆ Horsepower</li> <li>◆ VFD</li> </ul> </li> </ul>	1 Lava Rock / Wood Chip 800 3 3  1 7 10  1 5,000 9 10 Yes

Parameter Design Year	Design Parameter 2028
<b>ATAD ODOR CONTROL BIOFILTER (continued)</b>	
■ Anticipated System Performance	
◆ Ammonia Removal, %	95
◆ Methyl Mercaptans Removal, %	95
◆ Dimethyl Sulfide Removal, %	95
<b>SLUDGE STORAGE</b>	
■ Start-Up Sludge Volume @ 5% Solids, gpd	17,801
■ Design Sludge Volume @ 5% Solids, gpd	23,453
■ Sludge Storage Volume Needed, mg	
◆ Startup	3.2
◆ Design	4.2
■ Existing Tank Volume, mg	2.4
■ Existing Digesters Storage, mg	0.2
■ New Sludge Storage Tank	
◆ Diameter	81
◆ Sidewater Depth	42
◆ Volume, mg	1.6
<b>SECONDARY EFFLUENT PUMPS</b>	
■ Number Of Units	4 existing
■ Capacity Each Unit, gpm	3,819
<b>TERTIARY FILTERS (Removed From Service)</b>	
■ Number Of Units	6 existing
■ Dimensions Each Unit, feet	
◆ Length	20
◆ Width	20
■ Total Surface Area, feet	2,400
■ Type Of Media	Anthracite
■ Size Of Media, mm	1.8 to 2.0
■ Filtration Rate, gpm/square foot	
◆ @ 9 mgd	2.6
◆ @ 16.5 mgd	4.8
◆ @ 25 mgd	7.2
■ Waste Backwash Pumps	
◆ Number Of Units	2 existing
◆ Capacity, Each, gpm	850
■ Filter Backwash Pumps	
◆ Number Of Units	3 existing
◆ Capacity, Each, gpm	4,000
■ Filter Air Scour Blowers	
◆ Number Of Units	3 existing
◆ Capacity, Each, scfm	950

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